

### KUPPAM ENGINEERING COLLEGE

### DESIGN CONCEPTS IN RAFT FOUNDATION FOR HIGH RISE BUILDINGS

DEPARTMENT OF CIVIL ENGINEERING

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# **RAFT FOUNDATIONS**

#### Also known as Mat foundations

It is a continuous slab resting on the soil

 Extends over the entire footprint of the building thereby supporting the building and transferring its weight to the ground.

Best suitable when have a basement floor (High rise structures)

# **RAFT FOUNDATIONS**

### When do we need a raft foundation?

- Bearing capacity of the soil is low
- Load to be transferred to the ground is high
- Deep foundation becomes uneconomical
- More number of columns
- The total footing area is greater than 50% of the building area

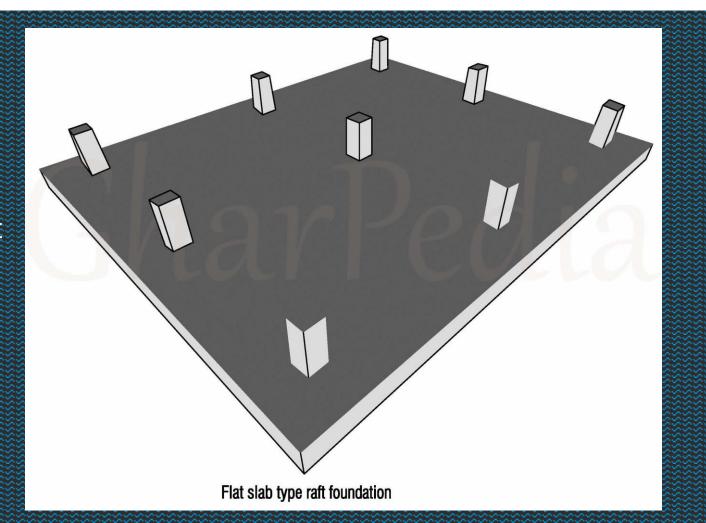
# **RAFT FOUNDATION**

#### Advantages

- Supports large number of columns
- Helps overcome differential settlement
- Distributes the loads on a wider area thereby not exceeding allowable bearing capacity
- The only shallow foundation to carry heavy loads
- Can carry lateral loads too
- Resists uplift pressure

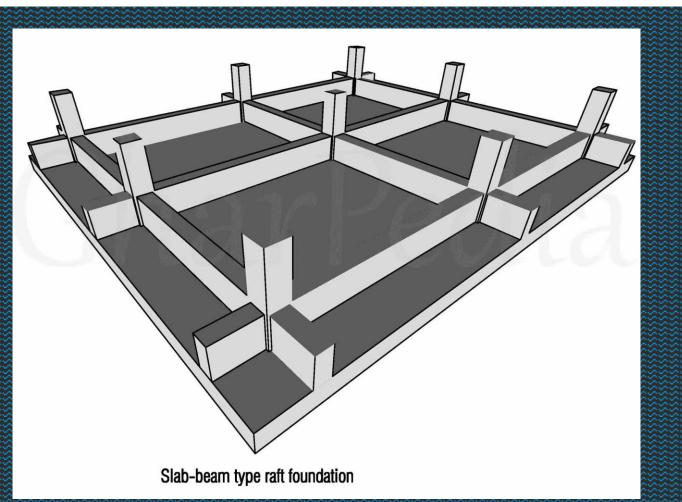
#### Flat Slab Type Raft Foundation

- Used when the columns are equally spaced
- Meaning uniform pressure throughout the slab.
- Slab has uniform thickness

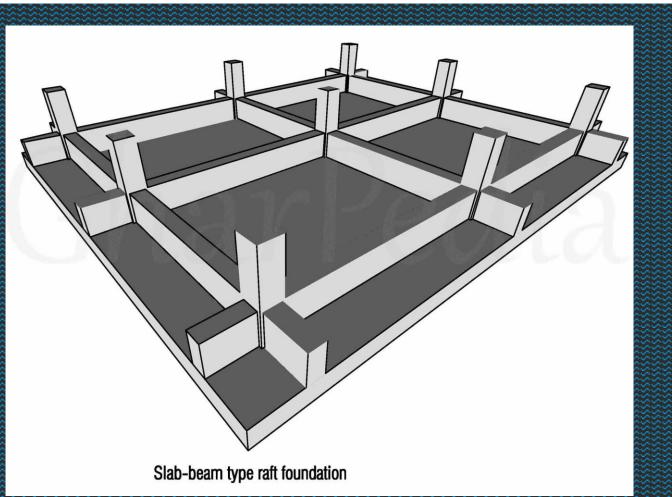


#### Slab-Beam Type Raft Foundation:

- Used when column loads are unequally distributed.
- To avoid excessive distortion of the structure as a result of variation in the load distribution on the raft. In this type of raft foundation beams are provided with the flat slabs.

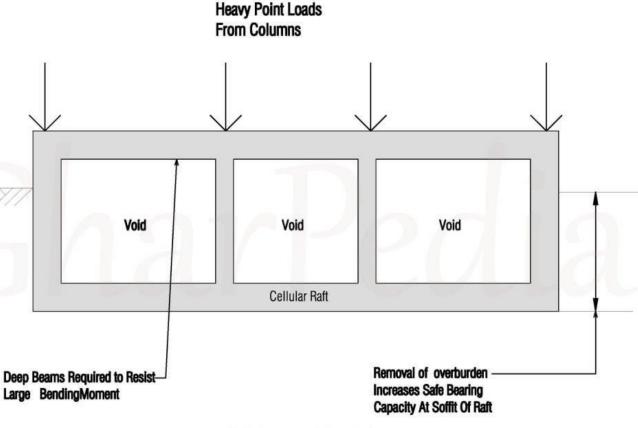


- Slab-Beam Type Raft Foundation:
- The beams add stiffness to the raft foundation.
- The foundation slabs are reinforced with two more steel meshes. One placed on the lower face and another at the upper faces of the raft foundation.
- The raft beams are reinforced with strong stirrups and bars placed at the upper and lower faces.



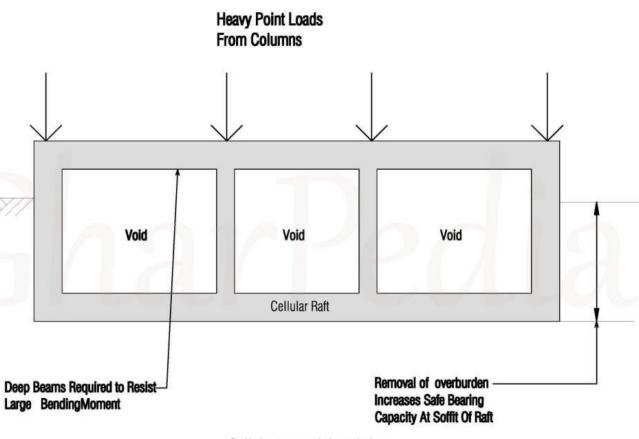
Cellular Type Raft Foundation:
In case of heavy structures on loose soil or when soil tends for uneven settlement, the thickness required will be more than 1m.

 In such case, cellular raft foundation is more preferable than ordinary raft foundation.



Cellular type raft foundation

- Cellular Type Raft Foundation:
- Consists two slabs where a beam is constructed of two slabs in both directions forming hollow cellular raft foundation.
- These foundations are highly rigid and more economical than other foundations in such type of poor soil

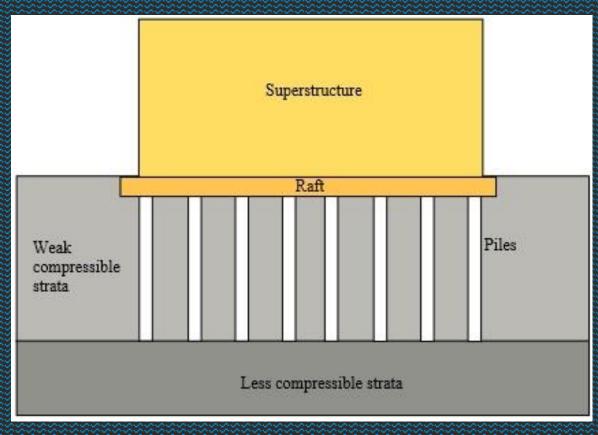


Cellular type raft foundation

#### **Piled Raft foundation**

 When the soil so weak that there is an excessive settlement of the raft slab then Raft slab is laid on the piles.

 Load is carried by the raft slab and settlement is resisted by piles



#### Two approaches

- Rigid foundation approach
- Flexible foundation approach

*Rigid Approach* - In rigid foundation approach, it is presumed that raft is rigid enough to bridge over non-uniformities of soil structure. Pressure distribution is considered to be either uniform or varying linearly.

(a) Inverted floor system (b) Combined footing approach

 In rigid rafts, differential settlements are comparatively low but bending moment and shear forces to which raft is subjected are considerably high

Flexible Approach

 In this approach, raft distributes the load in the area immediately surrounding the column depending upon the soil characteristics.

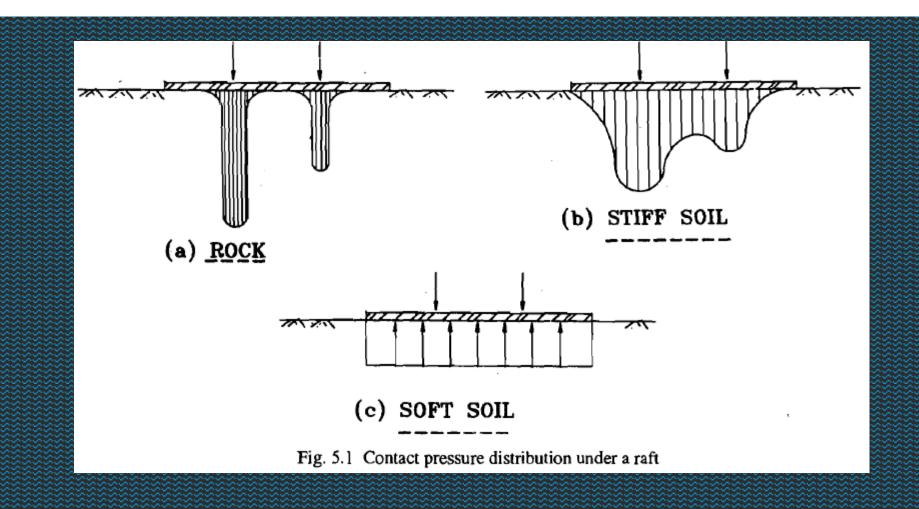
 Differential settlements are comparatively larger but bending moments and shear forces are comparatively low. Two approaches

(a) Flexible plate supported on elastic foundation, *i.e.*, Hetenyi's Theory

(b) Foundation supported on bed of uniformly distributed elastic springs with a spring constant determined using coefficient of sub-grade reaction. Each spring is presumed to behave independently, **i.e.**, Winklers's foundation

### Pressure distributed under the raft

- (1) The nature of the soil below the raft
- (2) The nature of the foundation, *i.e.*, whether rigid, flexible or soft
- (3) Rigidity of the super-structure
- (4) The quantum of loads and their relative magnitude
- (5) Presence of adjoining foundation
- (6) Size of raft
- (7) Time at which pressure measurements are taken



Settlement of Raft Slab

 The total settlement under the raft foundation can be considered to be made up of three components, *i.e.*,

#### S = *Sd*+*Sc*+*Ss*

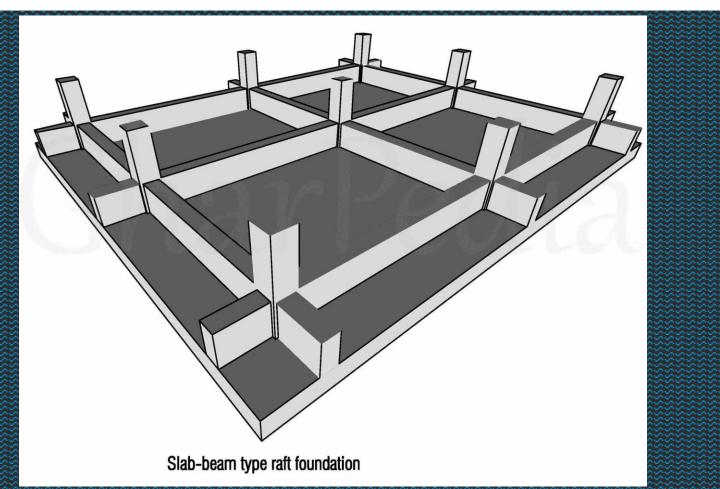
Where,

Sd is the immediate or distortion settlement Sc the consolidation settlement and **Ss** is the secondarycompression settlement

#### Design of Slab-beam type raft slab (Inverted slab)

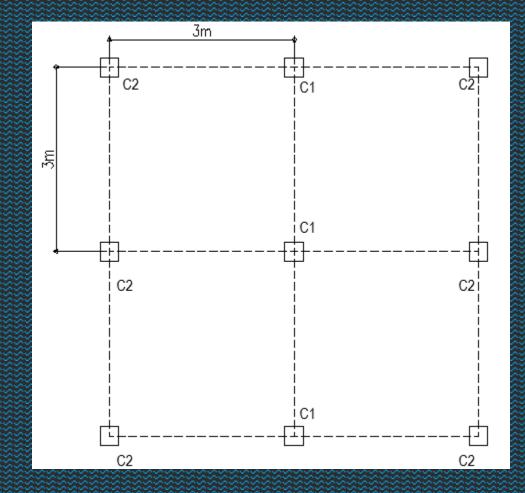
- The most common approach in Medium rise residential/commercial buildings
- The raft slab is designed as an inverted slab
- The uplift pressure is the loading on the slab, for which BM and SFs are calculated
- The raft beams stiffen the raft slab
- Raft beams are designed as inverted floor beams subjected to uplift pressure

Area of the slab will be equal to footprint of the building.
Cantilever portions are not always necessary, depends on the area required



 Example for discussion
 Design a raft footing for the foundation plan shown. Assume SBC 150kN/m<sup>2</sup>

C1 – 300x300 – 800 kN
C2 – 300x300 – 600 kN



#### Solution : -

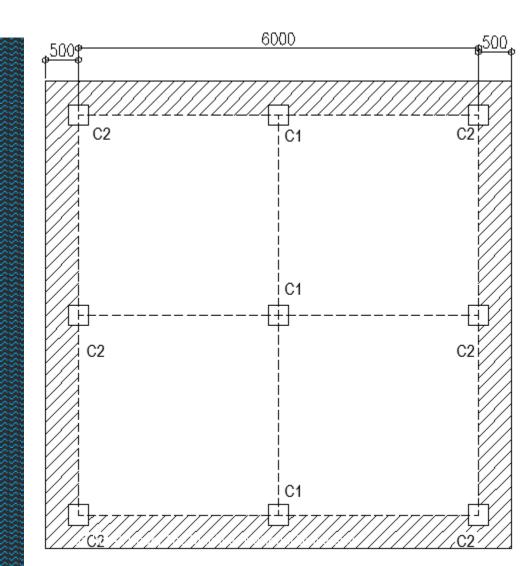
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- Calculation of column loads
- Load from column C1 = 3 x 800 = 2400 kN
- Load from column C2 = 6 x 600 = <u>3600</u> kN
- Total load on Foundation = 6000 kN
- Self weight of Foundation, 10% = 600 kN
  - Total load, w = 6600 kN
- Area of footing required, A = 6600/150 = 44 m<sup>2</sup>
   SBC = 150kN/ m<sup>2</sup>
- Footprint area of the grid = 6.3x6.3 = 39.69 m<sup>2</sup> < Area required.

Adopt a size of 7mx7m = 49m<sup>2</sup>

 Since, the grid cannot be changed, Extend the raft on the edges on all four sides as shown in the figure.

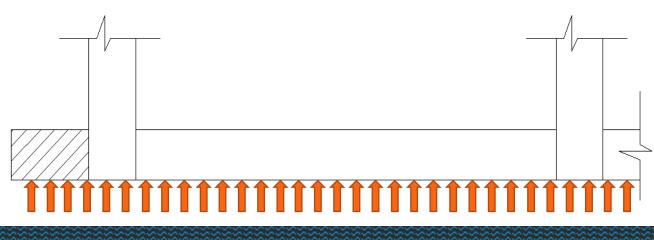
Now this area would be sufficient.



Net upward pressure,

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- p = Load of columns/Area provided
  - p = 6000/49 = 122.45 kN/m<sup>2</sup> < SBC



 Slab-beam type Raft slab is designed as inverted slab subjected to the Upward pressure 122.45 kN/m<sup>2</sup>

- The raft slab has interior panels as well as a cantilever portion as shown above.
- Hence we have to design both Interior panel and the cantilever portion.

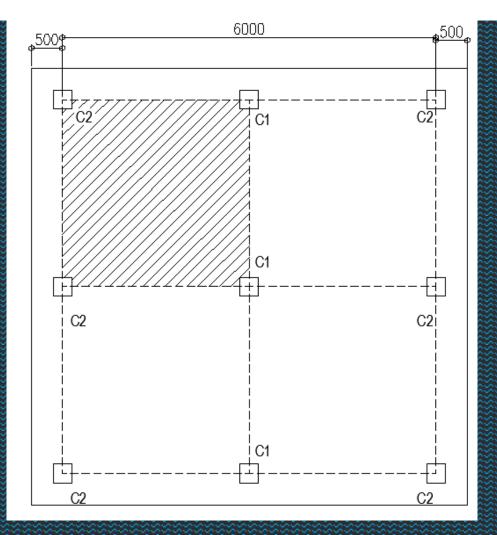
A) Cantilever Slab
Bending Moment, M = 1.5 x wl<sup>2</sup> / 2 = 1.5 x 122.45 x 0.35<sup>2</sup> / 2
M = 11.25 kN-m
B) Interior Panel
Ly = 3m, Lx = 3m
Ly/Lx = 1

• Referring Table 26, page 91, IS-456,



#### For Interior panels,

- Negative moment coefficients at continuous edge along short span  $\alpha x = -0.032$ , along longer span  $\alpha y = -0.032$
- Positive moment coefficients at midspan along short span  $\alpha x = -0.032$ , along longer span  $\alpha y = -0.032$



Bending Moments are calculated using the following formulae,

$$M_{x} = \alpha_{x} w l_{x}^{2}$$
$$M_{y} = \alpha_{y} w l_{x}^{2}$$

Negative bending moment, Mx = My = 0.032 x1.5x 122.45 x 3<sup>2</sup> = 52.89 kN-m (at supports)

Positive bending moment, Mx = My = 0.024x1.5x122.45x 3<sup>2</sup> = 39.67 kN-m (at Midspan)

- Depth required, Mu = 0.138fck bd<sup>2</sup>
- 52.89x10<sup>6</sup> = 0.138x20x1000d<sup>2</sup>
- d = 138.43 mm
- Adopt d = 150mm and a cover of 50mm, D = 200mm
- Area of steel required,

For negative moment, Mu = 0.87 \*fy \*Ast (d-0.42Xu,max)

- $52.89 \times 10^6 = 0.87 \times 415 \times \text{Ast} (150 0.42(0.48 \times 150))$
- Ast = 1223.19 mm<sup>2</sup>

Using 16 dia bars, spacing required = 201x1000/1223.19 = 164.4mm

Place T16 @ 150mm c/c

Area of steel provided, Ast = 1340 mm<sup>2</sup>

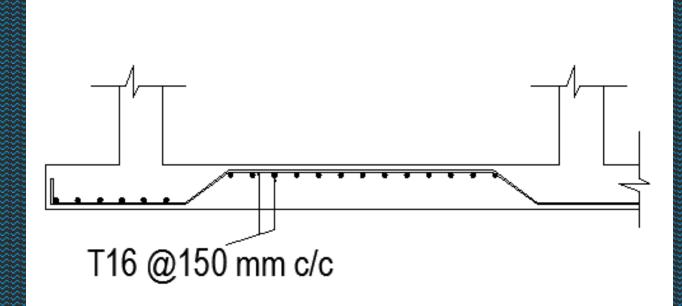
Similarly for positive moment,

Ast = 917.45 mm<sup>2</sup>

Using 16 dia bars, spacing required = 201x1000/917.45 = 219.08mm

Place T16 @ 150mm c/c

Area of steel provided, Ast = 1340 mm<sup>2</sup>



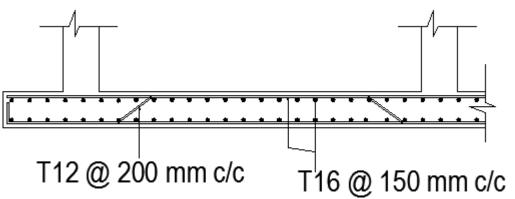
#### **Check for deflection**

- Steel stress of service, fs = 0.58 \* fy \* 1223.19/1340 = 219.72 N/mm<sup>2</sup>
- Percentage steel provided is 0.67
- Referring fig.4, IS456, Modification factor is 1.4
- Minimum depth, d min = 3000/26/1.4 = 82 mm
- Allowable s/d ratio = 1.4x26 = 36.4
- Provided s/d ratio = 3000/200 = 15

Hence the section safe in deflection

#### Check for Shear

Shear force,  $V_{U} = 122.45 \times 3/2 = 183 \text{kN}$ Shear stress, Tv = Vu/bd = 1.22N/mm<sup>2</sup> Now, 100Ast/bd = 0.8933, Referring to table 19, Design shear strength, k\*Tc = 1.2 x 0.6 = 0.72 N/mm<sup>2</sup> Shear reinforcement is required Let's provide bent-up bars, Area of shear reinforcement, for Vus = 1.22 - 0.72 = 0.5 N/mm<sup>2</sup> Asv = Vus/(σsv\*Sinα) = 0.5x1000x150/(230\*sin45) = 461.15 mm2 Provide T12 @ 200mm c/c

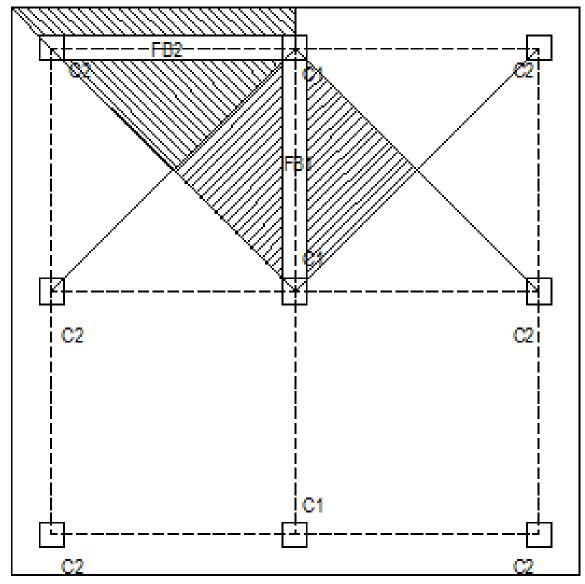


#### Check for cracking

- The steel provided is more than 0.12% of gross area
- Spacing is less than 3d
- Hence the section is safe

#### Design of Raft Beams

- Uplift pressure 122.45 kN/m2
- Load carried by foundation/raft beams is shown in the figure beside.
- FB1 carries the load distributed on two triangles,
- FB2 carries the load distributed on one triangle and a portion of cantilever



#### Design of FB1

- Area of loading = (0.5x3x3)x2 = 9 m<sup>2</sup>
- Loading on the beam, w = 122.45x9/3 = 367.35kN/m
- Moment at supports, Mu = 1.5xwl<sup>2</sup> / 10 = 1.5 x 367.35 x 3<sup>2</sup> / 10 = 495.92 kN-m (Table-12)
- Moment at mid span, Mu = 1.5xwl<sup>2</sup> / 12 = 1.5 x 367.35 x 3<sup>2</sup> / 12 = 413.27 kN-m
- Depth of the beam, d min, Mu = 0.138fckbd<sup>2</sup>, assuming b = 400mm
- d min = 670.22 mm,
- Adopt d = 720 mm, cover = 30mm
- Over all depth D = 750mm

#### Design of FB1

Mu lim will be greater than Mu

Beam can be designed as singly reinforced

$$M_{\rm u} = 0.87 f_{\rm y} A_{\rm st} d \left( 1 - \frac{A_{\rm st} f_{\rm y}}{b d f_{\rm ck}} \right)$$

Ast reqd = 2322.17 mm<sup>2</sup> • Provide 8-T20 • Ast Provided = 2512mm<sup>2</sup>

Design of FB1 Check for shear • Shear Force, Vu = 1.5x367.35x3x0.6 =991.84kN (Table 13, SF Co-efficient) τv = 661.23x1000/(300x800) = 4.132 N/mm<sup>2</sup> 100Ast/bd = 0.872 • τc = 0.59 N/mm<sup>2</sup>, τc max = 2.8 N/mm<sup>2</sup>

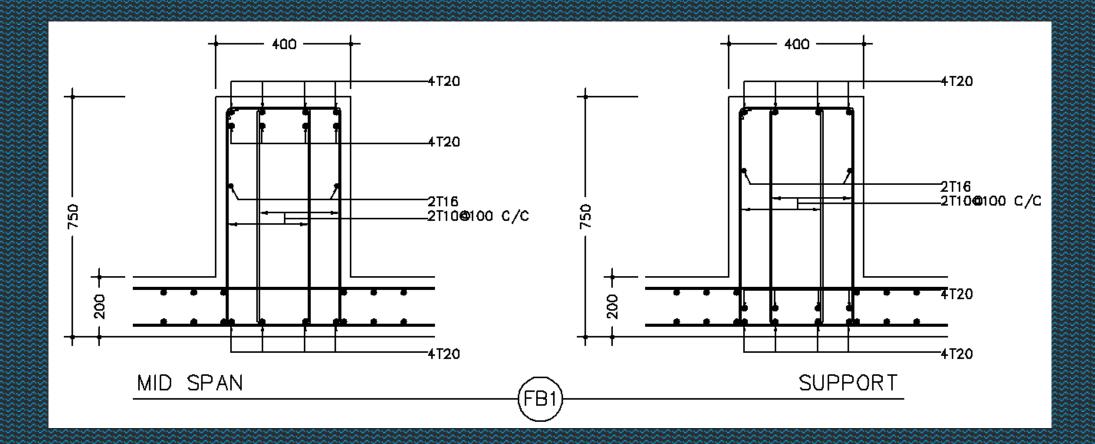
Design of FB1

Section requires shear reinforcement Vc = 0.59x400x720 = 169.92kN Shear to be resisted, Vus = Vu – Vc = 821.9kN

S = 0.87\*fy\*As\*d/Vus = 0.87x415x4x78.55x800/821920 = 110.42mm c/c

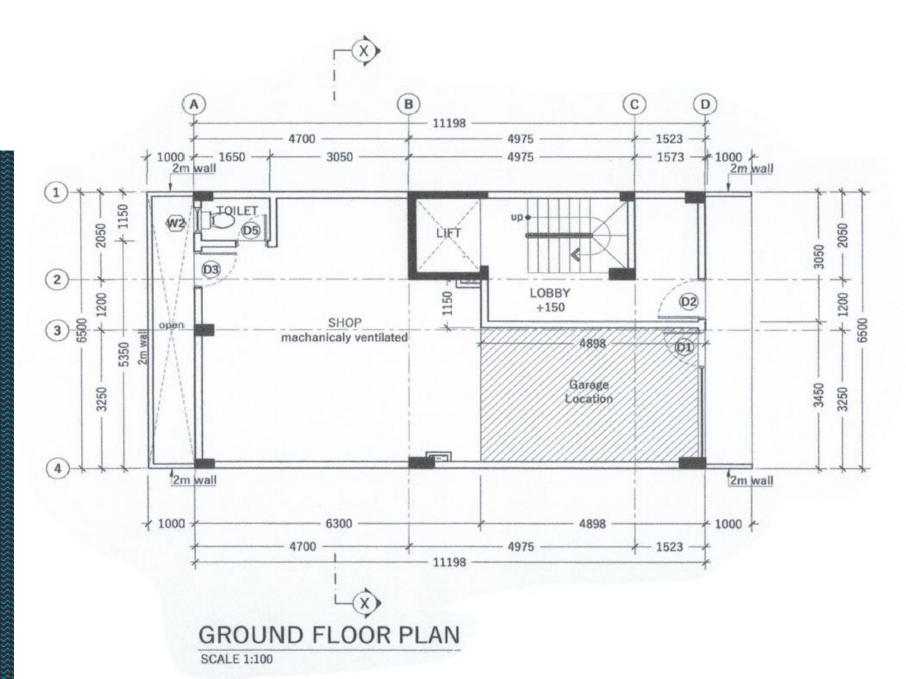
Provide 4-legged stirrups at 100 mm c/c

#### Structural details of FB1



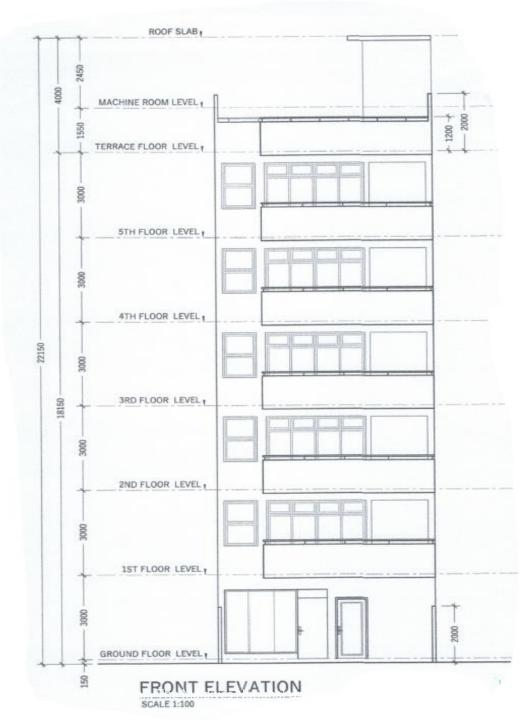
Design of Raft
 foundation for a
 6 floor building

Project -1



Project -1

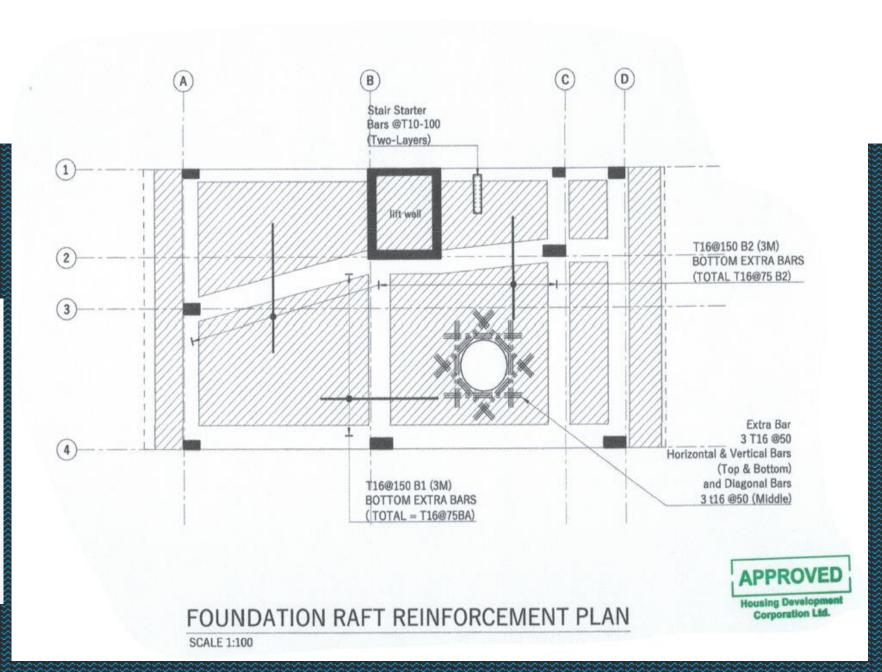
• Design of Raft foundation for a 6 floor building.



#### Project -1

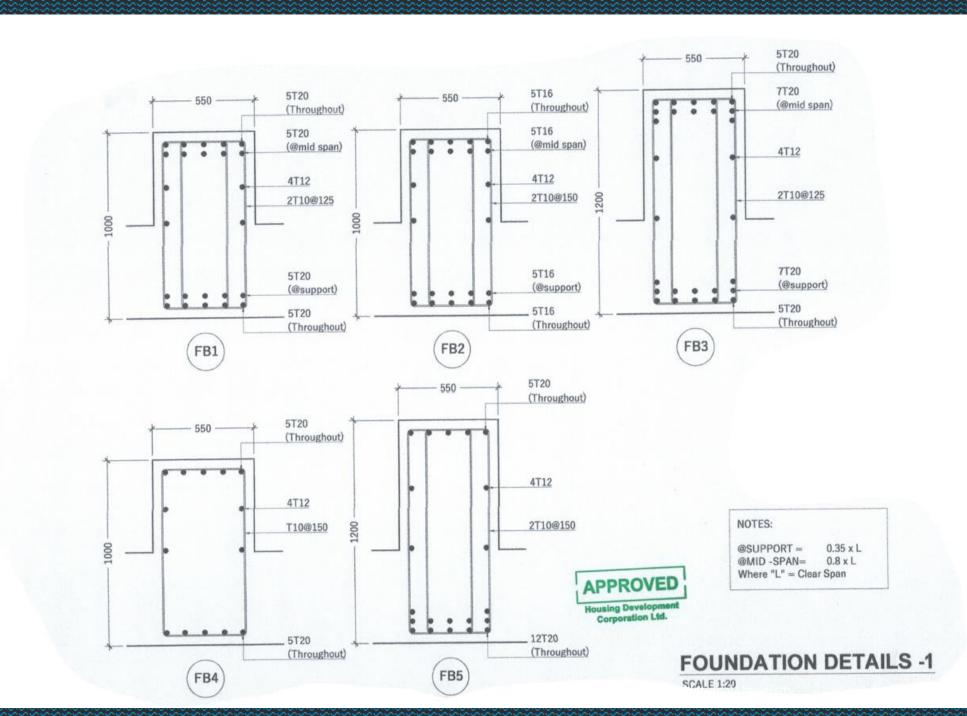
Details of Raft foundation

NOTES: RAFT THICKNESS = 500MM TOP & BOTTOM RFMT: T16 @150 B/W THROUGHOUT NOT SHOWN BOTTOM EXTRA BARS PROVIDE AS SHOWN PARRIED BOTTOM EXTRA BARS AS SHOWN = 50MM COVER TO FOUNDATION = 30-35MM COVER TO BEAMS = 40MM COVER TO COLUMNS = 450LAPS (Ø=BAR DIAMETER) = 1:2:3CONCRETE MIX



Project -1

 Design of Raft foundation for a 6 floor building.



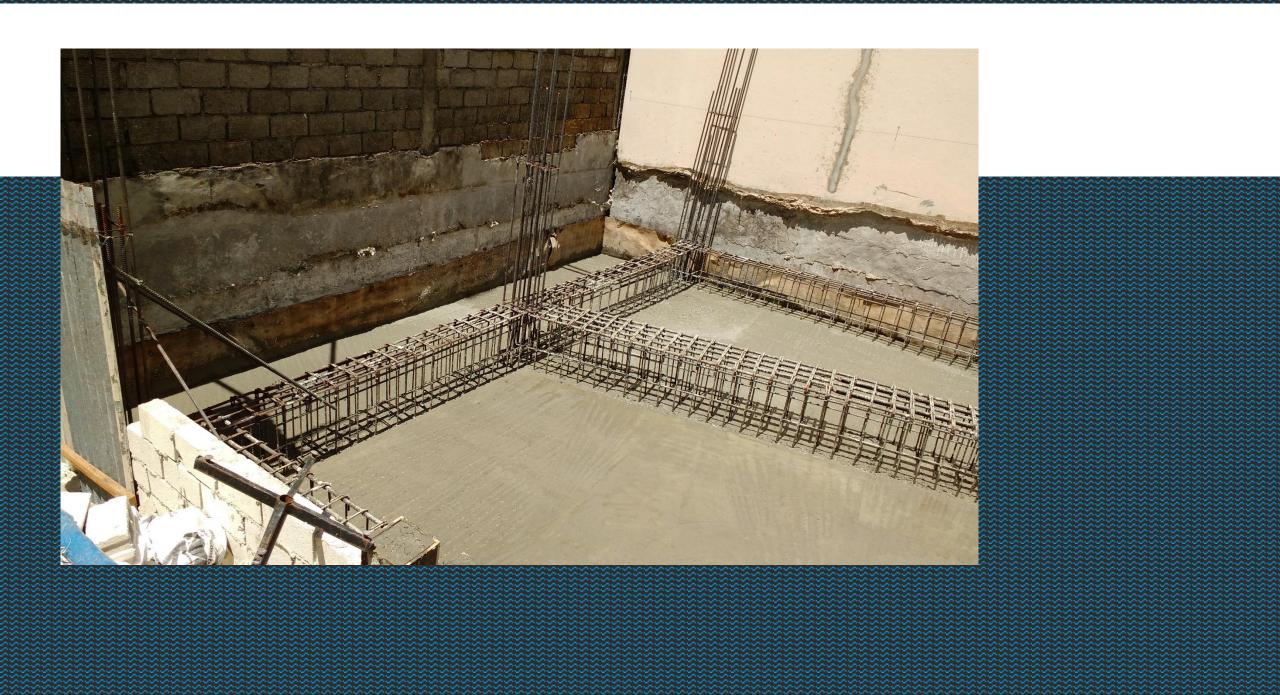
















Project-2

 Design of raft foundation for a 11 story Residential Building

